APPENDIX F

Soils Information

The following information and recommendations are from the Soils Section of the Iowa DOT's Office of Design.

A licensed engineer must certify that the bridge berms have been reviewed for long-term stability and provide a minimum slope stability factor of safety of at least 1.3. The engineer should investigate slope stability issues and settlement issues, and provide foundation recommendations or discussions. The engineer should also design any remedial measures that are necessary to address any geotechnical problems.

At least one boring per foundation (i.e. piers and abutments) must be tested by using standard penetration methods so that the 1994 Foundation Soils Information Chart booklet may be used to check foundation type and design. Tables 1.1 and 1.2 from this publication are shown on pages F-3 and F-4 of this appendix. A complete copy of this publication can be obtained by calling the Iowa DOT Office of Design, Soils Design Section, at (515) 239-1026.

Also included in this appendix is a sample Bridge Soil Profile Sheet (see page F-5).

Contact the Iowa DOT Office of Construction for general assistance pertaining to wave equation analysis. Please note however, that the Office of Construction is not available to perform wave equation analysis for city or county projects.

Factors to consider when selecting pile type:

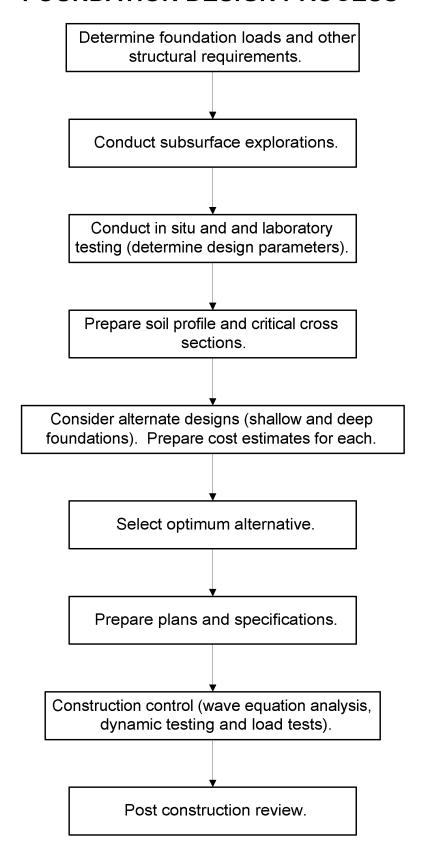
- 1. Displacement piles may not be drivable in materials having N-values in excess of 25.
- 2. Displacement piles may not be drivable if identifiable boulder layers are present.
- 3. Displacement piles may not be drivable through "dry" sands.
- 4. Steel H piles with driving shoes may be required if sloping bed rock surfaces are to be penetrated.
- 5. Steel H piles with driving shoes may be required if significant layers of N > 100 are to be penetrated.

Selecting a foundation layer:

- 1. End all piles in a non-compressible layer.
- 2. Be sure the required design bearing is obtained below the potential erosion/scour depth.
- 3. Piles must be well seated in the foundation layer.
- 4. Minimum pile length in contact with the ground is ten feet.

The flow chart on the following page illustrates the geotechnical foundation investigation, selection and design process for bridges.

FOUNDATION DESIGN PROCESS



	DRIVEN PILE		NDATIC	FOUNDATION SOILS INFORMATION CHART, ENGLISH UNITS	TION C	HART	ENGLI	SH UN	ITS		
				ESTIMATED ALLOWABLE END BEARING VALUES IN TONS	BLE END	BEARII	IG VALL	JES IN T	ONS.		
	BLO	BLOW COUNT		Factor of Safety = 2.0	0						
	ż	N - VALUE	WOOD	STEEL 'H'	ຽ	CONCRETE	ш	(C)	STEEL PIPE PILE	IPE PILE	
SOIL DESCRIPTION	MEAN	RANGE	PILE	10 12 14	12	14	16	10	12	14	<u>2</u>
GRANULAR MATERIAL	> 10										
Medium Sand	15		8		15	21	27	ω	12	16	27
Fine Sand	15		8	*	12	21	27	∞	12	16	27
Coarse Sand	20		-		21	29	37	1	16	22	36
Gravelly Sand	21		11		21	29	37	11	16	22	36
	25		14								
		25 - 50		1 000 - 2 000 psi							
		50 - 100		2 000 - 4 000 psi				1.4 × 'N'	ż		
		100 - 300	*	4 000 - 8 000 psi				(tons	(tons/sq.ft.)		
		> 300		9 000 psi		**					
Bedrock		100 - 200		6 000 psi							
-											
COHESIVE MATERIAL						. 1	(•	•	(•
	12	10 - 50	4		_	10	<u></u>	4	ဖ	ω	<u>.</u>
•	20		9	500 psi	=	16	21	7	တ	<u>ლ</u>	21
	25		ω	1 000 psi	15	21	27	∞	12	16	27
	20		20	2 000 psi	29	41	53	14	24	32	53
	100		**	3 500 psi				7.5			

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0.5 sq. ft.	limensic
is based on the end area $= 0$	different tip o
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End Bearing for we	Bearing shall b
NOTE:	

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^{**} Usage is NOT recommended in heavy shaded area.

u io	'U' DII E END ABEA
1	
SIZE	IN SQUARE INCHES
10X42	12.4
10X57	16.8
12X53	15.5
12X74	21.8
14X72	21.4
14X89	26.1

	DRIVEN	IVEN PILE FOUNDATION SOILS INFORMATION CHART, ENGLISH	NDATI	OS NO	LS INF	ORMA	TION (HART	, ENGL	ISH UN	UNITS		
	<u> </u>	BI OW COUNT	EST	ESTIMATED ALLOWABLE BEARING VALUES FOR FRICTION PILE IN T/FT Factor of Safety = 2.0) ALLO	WABLE 2.0	BEARIN	G VALI	JES FOR	FRICTI	ON PILE	IN T/F	ے
	Z	VALUE	doow	S	STEEL "H	-	Ö	CONCRETE	11		STEEL !	PIPE PILE	
SOIL DESCRIPTION	MEAN	MEAN RANGE	PILE	5	12	14	12	14	16	10	12	14	18
ALLUVIUM or LOESS											,	1	,
Very Soft Silty Clay		0-1	0.2	0.1	0.5	0.2	0.2	0.5	0.2	o. 1.	0.1	0.1	0.2
Soft Silty Clay	m	2.4	0.3	0.2	0.3	0.3	0.2	0.2	0.2	0,2	0.2	0.2	ი:ი
Stiff Silty Clay	ဖ	4 - 8	0.4	0.3	4.0	0.5	0.3	0.4	0.5	ი.ვ	0.3	9.0	0.5
Firm Silty Clay	11	7-15	9.0	0.5	9.0	7.0	9.0	0.7	0.8	4.0	0.5	9.0	7.0
Stiff Silt	မ	3-7	0.4	0.3	0.4	0.4	0.4	0.4	0.4	0.3	0.3	9.4	0.4
Stiff Sandy Silt	9	4-8	4.0	0.3	0.4	4.0	0.4	0.4	0.4	0.3	0.3	4.0	0.4
Stiff Sandy Clay	9	4 - 8	4.0	0.3	0.4	0.5	0.5	0.5	9.0	0.3	0.4	0.4	0.5
Silty Sand	8	3 - 13	0.3	0.3	0.3	4.0	0.4	4.0	4.0	0.2	0.2	0.3	0.4
Clayey Sand	13	6 - 20	0.5	0.4	0.5	0.7	9.0	9.0	0.7	0.4	0.5	0.6	0.7
Fine Sand	15	8 - 22	9.0	0.5	9.0	7.0	0.6	0.7	0.8	0.4	0.5	9.0	0.7
Coarse Sand	20	12 - 28	0.8	0.7	0.8	0.9	0.8	6.0	0.	0.5	9.0	0.7	0.9
Gravelly Sand	21	11 - 31	9.0	0.7	9.0	6.0	6.0	6.0	0:0	0.5	9:0	7:0	6.0
Granular Material	>40		1.0	1.0	1.2	4.1				0.8	1.0	:	1.4
						-							
GLACIAL CLAY	,			• (1	(1	(- (1	((Ċ
Firm Silty Glacial Clay	11	7 - 15		o.0). 	χ.ς χ.ς) . (χ. Ο (၁ (၁ ပ	ه د د د	ه د د	χ. Ο C
Firm Clay (Gumbotil)	12			9.0	7.0	8 0). (0.8 0.8	6.0 6.0	O.5	9 9 0	0.0 1	χ. Ο (
Firm Glacial Clay	=			0.7	8. 0	6.0	ω ; Ο ;	o ;	0.5	0.5	9 i) (0.5	o ;
		50,000,000		(0.8)	(1.0 (0.1	(1.1)	() ()	(1.1	(1.2)	(0.6)	(0.7)	(0.8)	(1.1
Firm Sandy Glacial Clay	13	9 - 15		0.7	9.6	6.0	8.0	6.6 6.5	0.5	9.50	9 i	\	n :
i		░,		(0.8) 0.3	() () ()	- (- (5 -	Ξ, Ξ,	7.7	6 5 6) (C)	(2.0 (2.0	- , - ,
Firm - Very Firm Gl. Clay	14	11 - 17) ;	χ. i	n :	o (- 3	7.5	ρ (c)	> 6 • 6	ο δ Ο •) - -
		8		(1.0)	(1.2)	(1.4)	(1.2)	(1.3)	(1.4)	(0.8)	(0.9)	() () ()	(S.)
Very Firm Glacial Clay	24	17 - 30		0.7	0.8	6.0	8.0	6.0		9.6	0.7	9.8 9.8	0.5
				(O.E.	(1.2)	(1 .4	(1:2)	(1.4)	(1.6)	(0.8)	(6.9)	0;C	(S.E.
Very Firm Sandy GI. Clay	22	15 - 30		0.7	0.8	6.0	0.8	6. 0		9.0	0.7	0.8	0
				(1.0	(1.2)	(1.4) 	(1.2)	(1.4)	(1.6)	(0.8)	(O.9)	(1.0	(1.3)
Cohesive or Glacial Material	>35		0.8	0.7	0.8	6.0	0.8	6.0	1.1	0.5	9.0	7.0	6.0
				(1:0	(1.2)	1.4)	(1.2)	(1.4)	(1.6)	(0.8)	0.0		(1.4)
					TABLE (7.							

For Double Entries: Upper Value is designated for an embedded pile within 30' of the natural ground elevation. Lower Value is designated for pile depths more than 30' below the natural ground elevation.

